

# C.3 Workshop – Track 2: Sizing Calculations and Design Considerations for LID Treatment Measures

Jill Bicknell, P.E., EOA, Inc.

Santa Clara Valley Urban Runoff Pollution Prevention Program

# Presentation Overview

---

- Determining Water Quality Design Flow and Volume (“ $Q_{BMP}$ ” and “ $V_{BMP}$ ”)
- Sizing Bioretention and Flow-Through Planters
- Sizing Pervious Paving and Infiltration Trenches
- Sizing Rainwater Harvesting Cisterns
- Sizing Non-LID Components

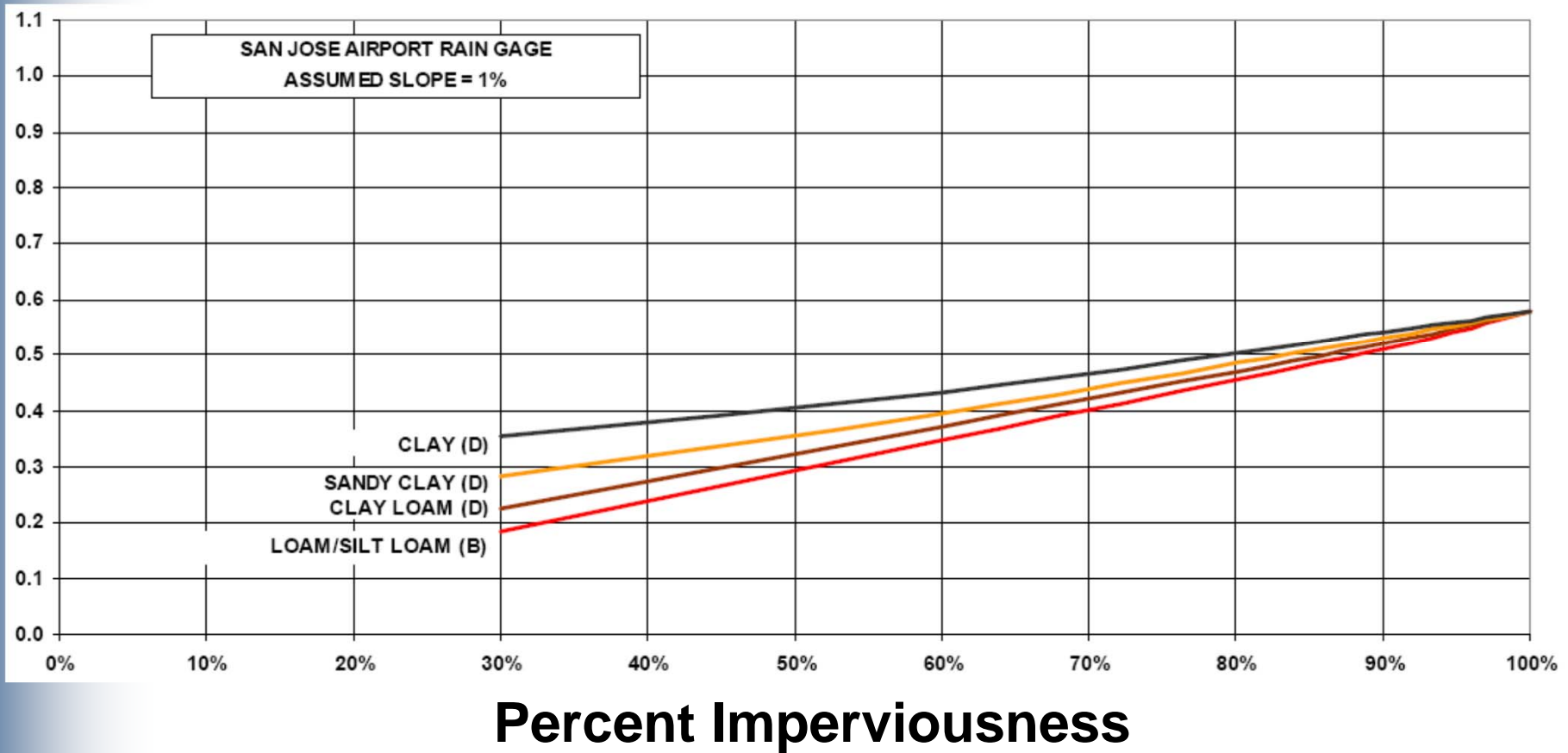
# C.3.d Sizing Criteria

---

- Volume-based sizing criteria:
  - URQM Method - use formula and volume capture coefficients in “Urban Runoff Quality Management”, WEF/ASCE MOP No. 23 (1998), pages 175-178
  - CASQA BMP Handbook Method - Determine volume equal to 80% of the annual runoff, using methodology in Appendix D of the CASQA BMP Handbook (2003) using local rainfall data
    - **Sizing curves specific to Santa Clara Valley provided in Appendix B of C.3 Handbook**

# Unit Basin Storage Volume for 80% Capture (inches)

## San Jose Rain Gage, 1% Slope

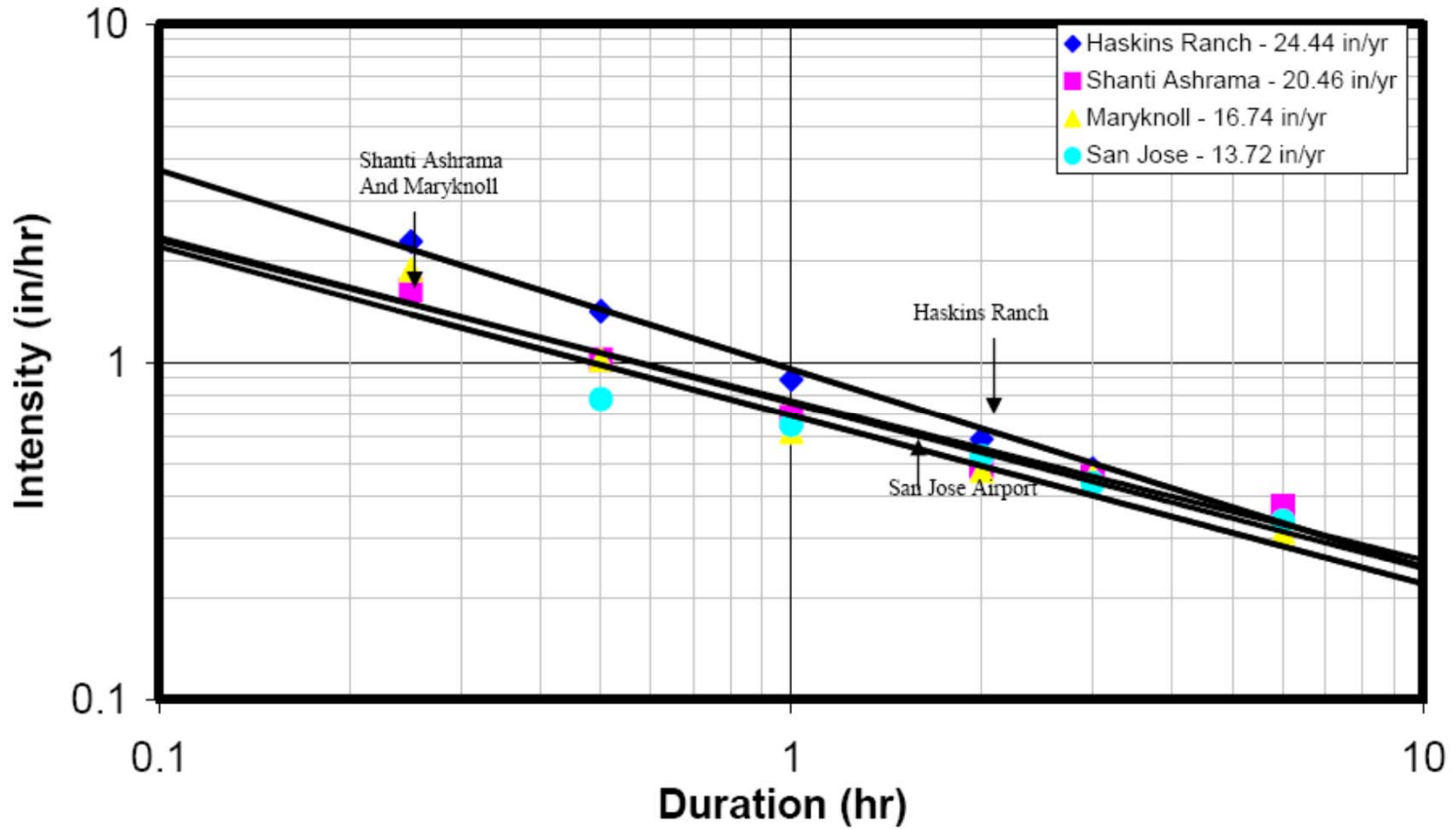


# C.3.d Sizing Criteria

---

- Flow-based sizing criteria:
  - Factored Flood Flow - 10% of the 50-year peak flow rate, determined using Intensity-Duration-Frequency curves published by the local flood control agency
  - Percentile Rainfall Intensity - Flow of runoff produced by a rain event equal to two times the 85th percentile hourly rainfall intensity
    - Data for Santa Clara Valley rain gages in Sizing Worksheets (Appendix B of C.3 Handbook)
  - Uniform Intensity - Flow of runoff resulting from a rain event equal to 0.2 inches per hour intensity

# Intensity-Duration-Frequency Curve (50-Year Return Period)



# C.3.d Sizing Criteria

- 85<sup>th</sup> Percentile Rainfall Intensity Data:

Reference Rain Gages	85 <sup>th</sup> Percentile Hourly Rainfall Intensity (in/hr)	Design Rainfall Intensity (in/hr)*
San Jose Airport	0.087	0.17
Palo Alto	0.096	0.19
Morgan Hill	0.12	0.24

\*Design rainfall intensity = 2 X 85<sup>th</sup> percentile hourly rainfall intensity

- By comparison, Uniform Intensity = 0.2 in/hr

# C.3.d Sizing Criteria

---

- Flow-based sizing criteria:
  - Simplified Sizing Approach – Variation of Uniform Intensity Method (0.2 in/hr)
    - The surface area of a biotreatment measure is sized to be 4% of the contributing impervious area
    - Based on a runoff inflow of 0.2 in/hr (assume equal to the rainfall intensity), with an infiltration rate through the biotreatment soil of 5 in/hr  
( $0.2 \text{ in/hr} \div 5 \text{ in/hr} = 0.04$ )
    - Conservative approach because does not account for surface ponding – good for planning purposes



# C.3.d Sizing Criteria

---

- **Combination Flow & Volume Design Basis:**
  - Treatment systems can be sized to treat “at least 80% of total runoff over the life of the project”
  - Option 1: Use a continuous simulation hydrologic model (typically not done for treatment measures)
  - Option 2: Show how treatment measure sizing meets both flow and volume-based criteria
    - Used for bioretention and flow-through planters
    - Appropriate where drainage area is mostly impervious

# Flow- or Volume-Based Sizing for Treatment Measures?

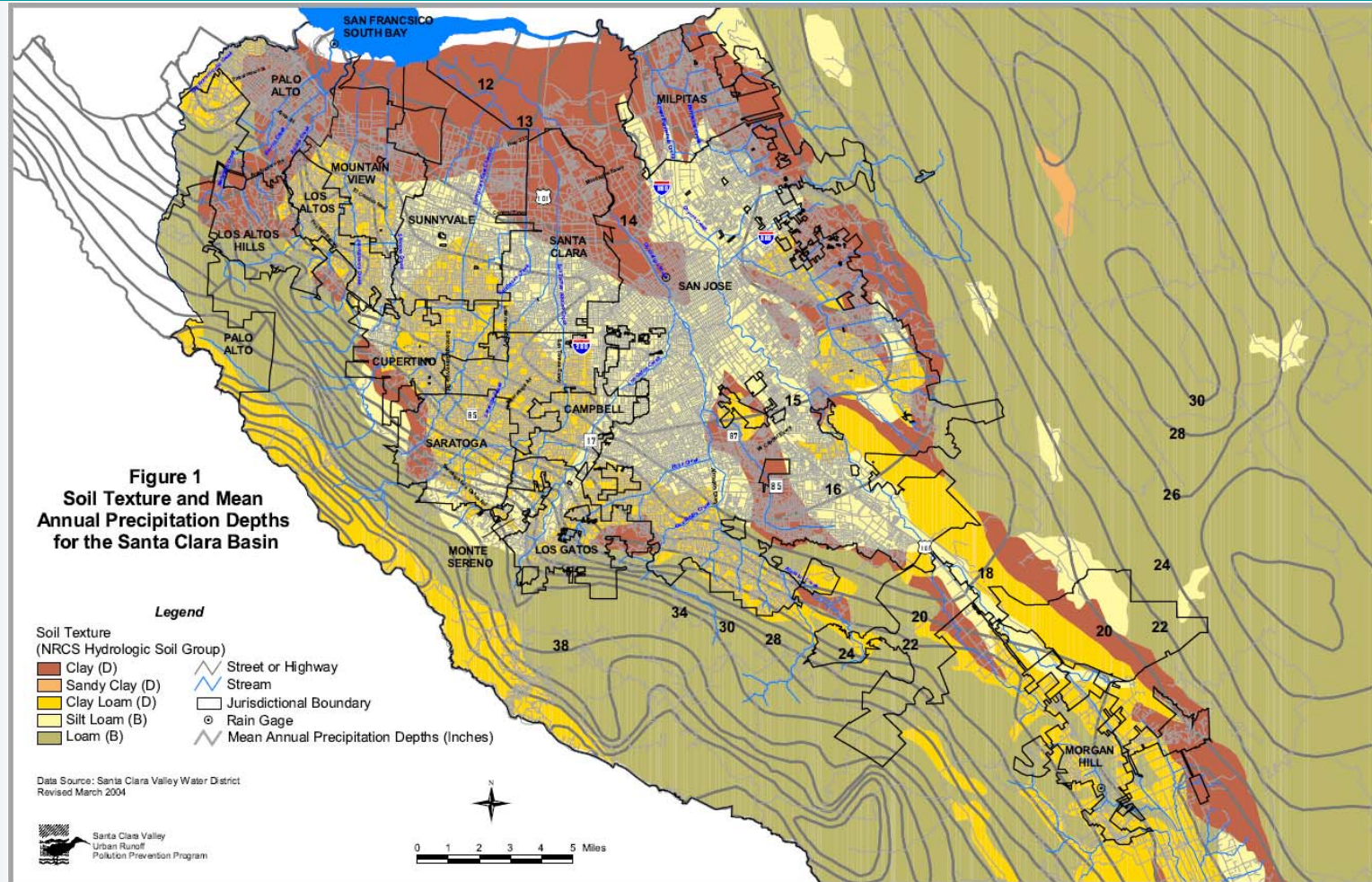
Table 5-1 Flow and Volume Based Treatment Measure Sizing Criteria		
Type of Treatment Measure	LID?	Hydraulic Sizing Criteria
Bioretention area	Yes	Flow- or volume-based or combination
Flow-through planter box	Yes	Flow- or volume-based or combination
Tree well filter	Yes	Flow-based
Infiltration trench	Yes	Volume-based
Subsurface infiltration system	Yes	Volume-based
Rainwater harvesting and use	Yes	Volume-based
Media filter	No	Flow-based
Extended detention basin	No	Volume-based

# Sizing Criteria Worksheets

---

- Appendix B of SCVURPPP C.3 Handbook
  - Worksheets for determining water quality design flow and volume
  - Figure B-1: Soil Texture and Mean Annual Precipitation (MAP) Depths
  - Figures B-2 – B-7: Unit Basin Storage Volume for 80% Capture (3 gages, 1% and 15% slopes)
  - Figure B-8: Intensity-Duration-Frequency Curves for 50-year Return Period (4 gages)

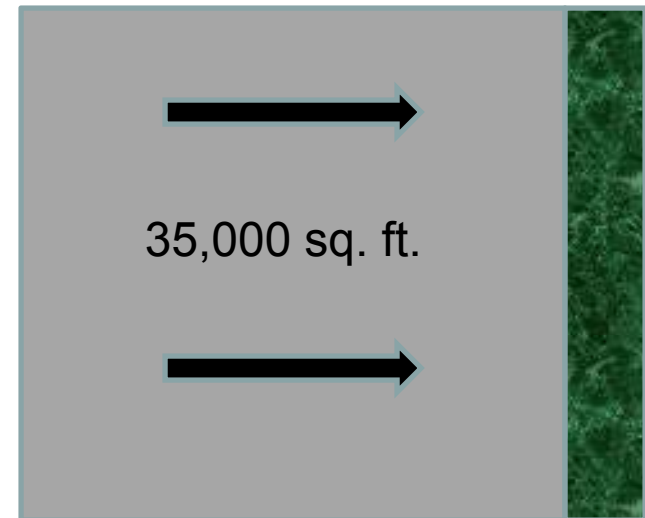
# Figure B-1: Soil Texture and Mean Annual Precipitation (MAP) Depth



# Sizing Example #1

- Parking lot in Santa Clara

- Area = 35,000 sq. ft. (0.80 acres)
- 100% impervious
- Slope = 1%
- Mean annual precipitation (MAP) = 15 inches



- Use the sizing worksheets

to determine  $Q_{BMP}$  and  $V_{BMP}$

- **Answer:  $V_{BMP} = 1,819$  cu. ft.;  $Q_{BMP} = 0.103$  cfs**

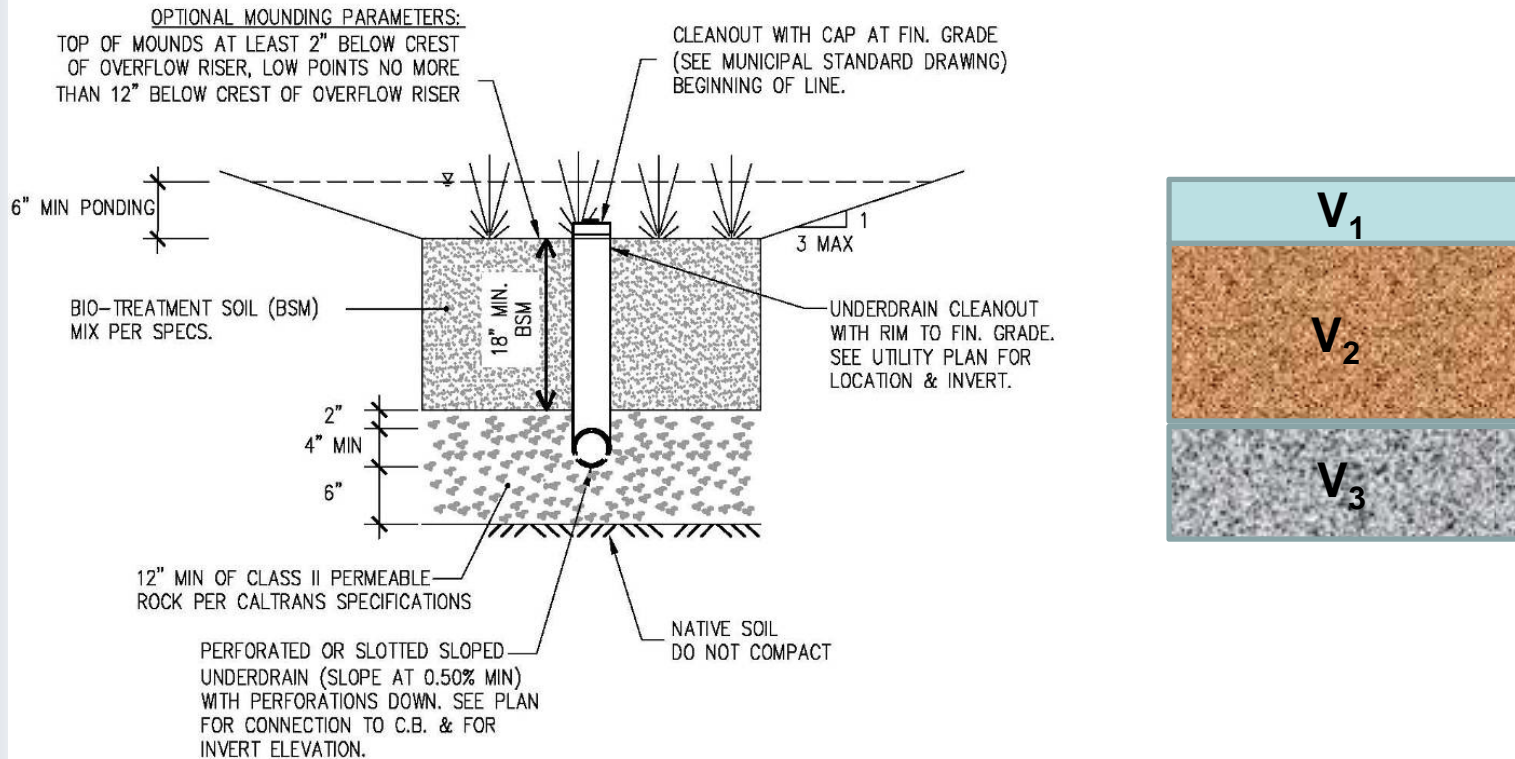
# Sizing Bioretention Facilities

---

- Simplified Sizing Approach
  - Surface area is 4% of contributing impervious area
  - Does not consider storage in surface ponding area
- Volume Based Approach
  - Store  $V_{BMP}$  in just surface ponding area
  - Store  $V_{BMP}$  in ponding area, soil media & drain rock
- Combination Flow and Volume Approach
  - Compute both  $Q_{BMP}$  and  $V_{BMP}$
  - Route through facility, allowing ponding



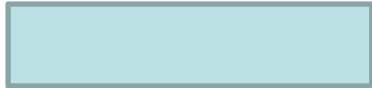
# Sizing Bioretention Facilities: Volume-Based Approach



# Sizing Bioretention Facilities: Volume-Based Approach

Method 1: Store entire volume in surface ponding area

$V_1$



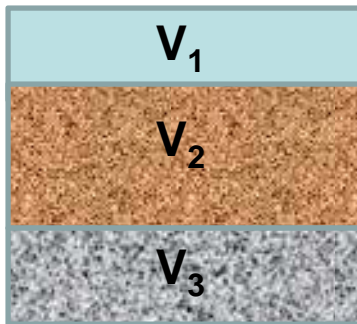
Depth (ft)	Porosity	Volume per sq. ft. (cubic feet)
0.5	1.0	0.5

$$\text{Surface Area} = V_{\text{BMP}} \text{ (cu.ft.)} \div 0.5 \text{ cu.ft./sq.ft.}$$



# Sizing Bioretention Facilities: Volume-Based Approach

Method 2: Store volume in ponding area and media



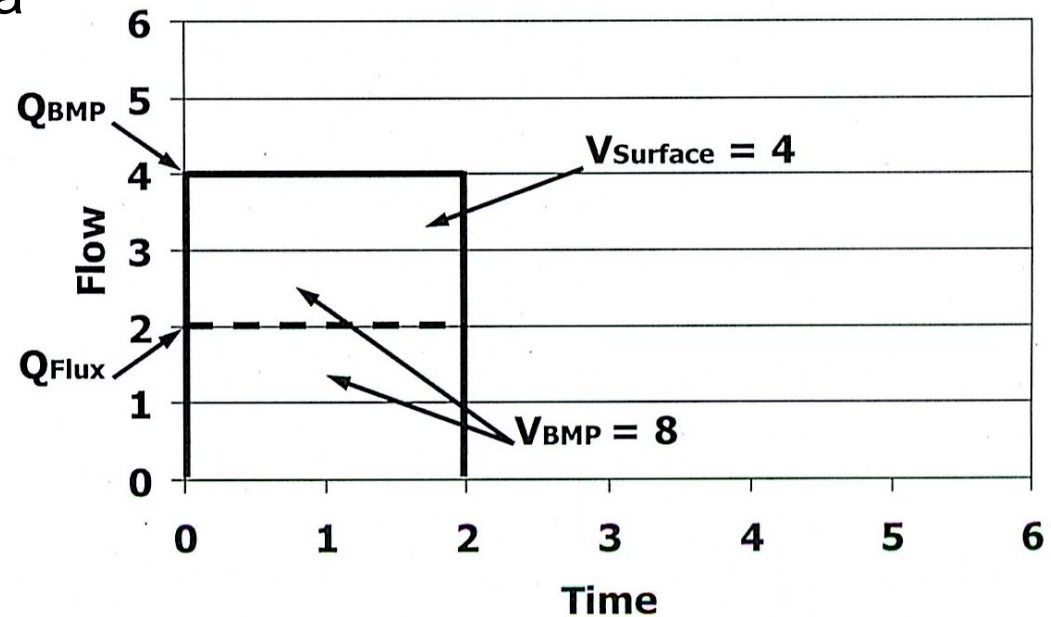
Depth (ft)	Porosity	Volume per sq. ft. (cubic feet)
0.5	1.0	0.5
1.5	0.30	0.45
0.5*	0.40	0.20
<b>Total</b>		1.15

\*Depth below bottom of underdrain

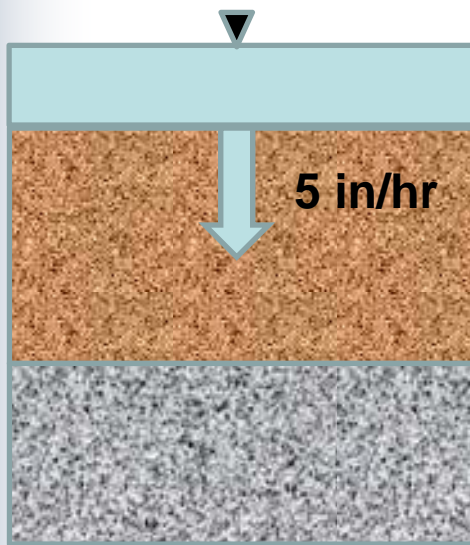
$$\text{Surface Area} = V_{\text{BMP}} \text{ (cu.ft.)} \div 1.15 \text{ cu.ft./sq.ft.}$$

# Sizing Bioretention Facilities: Flow & Volume Approach

- “Hydrograph Approach”
  - Runoff is routed through the treatment measure
  - Assume rectangular hydrograph that meets both flow and volume criteria

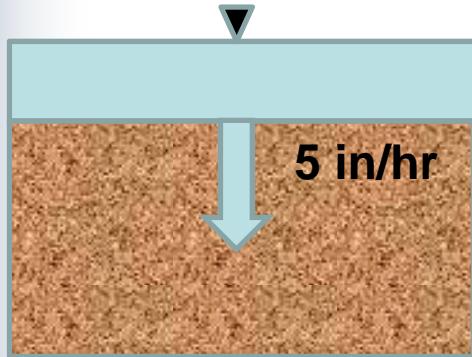


# Sizing Bioretention Facilities: Flow & Volume Approach



- Determine  $V_{BMP}$
- Assume constant rainfall intensity of 0.2 in/hr continues throughout the storm (rectangular hydrograph)
- Calculate the duration of the storm by dividing the Unit Basin Storage by the rainfall intensity
- Calculate the volume of runoff that filters through the biotreatment soil at a rate of 5 in/hr over the duration of the storm and the volume that remains on the surface

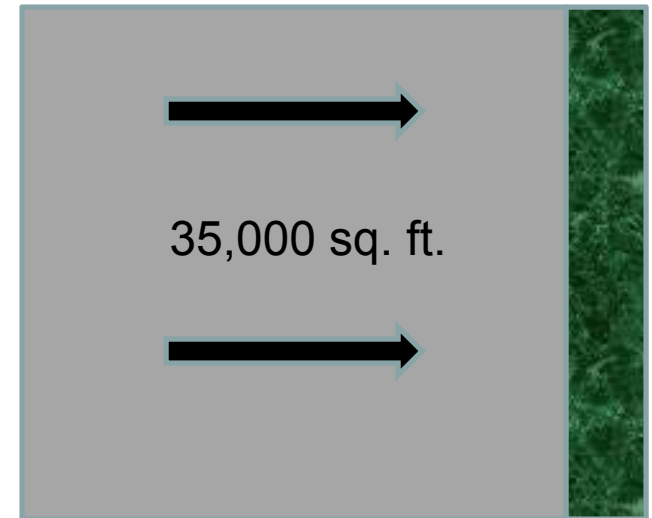
# Sizing Bioretention Facilities: Flow & Volume Approach



- To start the calculation, you have to assume a surface area “ $A_S$ ” -- use 3% of the contributing impervious area as a first guess
- Determine volume of treated water “ $V_T$ ” during storm:  
$$V_T = A_S \times 5 \text{ in/hr} \times \text{duration (hrs)} \times 1 \text{ in}/12 \text{ ft}$$
- Determine volume remaining on the surface “ $V_S$ ”:  
$$V_S = V_{BMP} - V_T$$
- Determine depth “ $D$ ” of ponding on the surface:  
$$D = V_S \div A_S$$
- Repeat until depth is approximately 6 inches

# Sizing Example #1, continued

- Parking lot in Santa Clara
  - Area = 35,000 sq. ft. (0.8 acres)
  - 100% impervious
  - $V_{BMP} = 1,819$  cu. ft.
  - UBS Volume = 0.63 in.
- Use the combination flow and volume sizing worksheet to determine the bioretention surface area
- **Answer: 1,000 sq. ft. (depth of 0.5 ft.)**



# Sizing Bioretention Facilities: Comparison of Methods

Example: 35,000 sq. ft. parking lot in Santa Clara  
MAP= 15 inches, 100% impervious  
 $V_{BMP} = 1,819$  cu. ft. (80% of annual runoff)

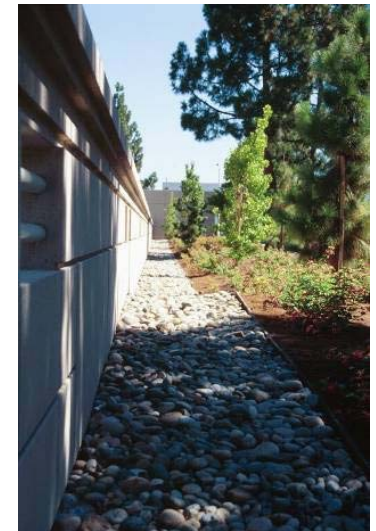
Sizing Method	Surface Area (sq. ft.)
Simplified Method (flow-based)	1,400
Volume ponded on surface	3,638
Volume stored in unit ( $V_1+V_2+V_3$ )	1,580
Combination flow & volume	1,000



# Sizing Pervious Paving and Infiltration Trenches

## ■ General Principles

- Store the WQD Volume in void space of stone base/subbase and infiltrate into subgrade
- Surface allows water to infiltrate at a high rate
- Any underdrains must be placed above the void space needed to store and infiltrate the WQD volume



# Sizing Pervious Paving and Infiltration Trenches

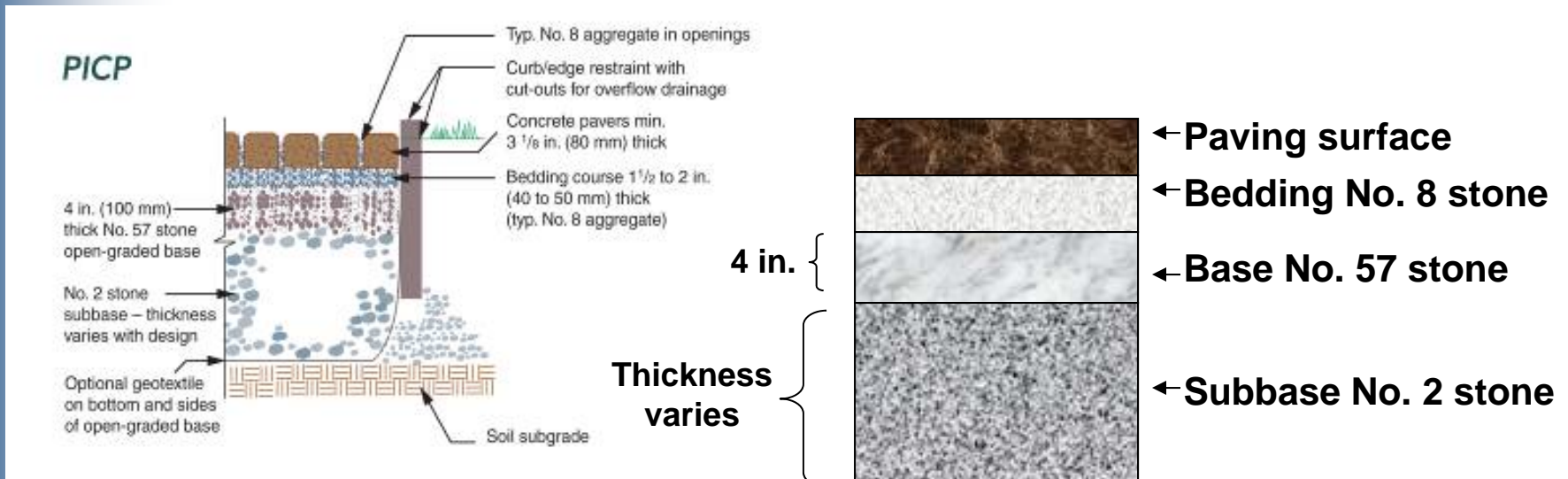
---

- Pervious Paving
  - May be self-treating area or self-retaining area (accept runoff from other areas)
  - Can only be considered a “pervious area” if stone base/subbase sized to store the WQD volume
  - Can work where native soils have low infiltration rates (stored water depths are relatively small)
  - Surface area is usually predetermined
  - Base and subbase thickness usually determined by expected traffic load and saturated soil strength
  - Slope should be  $\leq 1\%$  (or use cutoff trenches)



# Pervious Paving

## Typical Section



- Base and subbase layers available for water storage
- Both typically have 40% void space

# Pervious Paving

---

- Approach to Sizing Pervious Paving

- Self-Treating

- Check the depth of the WQD volume in base/subbase:  
UBS volume (in.)  $\div$  0.40 = Depth (in.)

Example: UBS volume = 1.0 in., depth = 2.5 in.  
(Minimum depth for vehicular traffic is 10 in.)

- Check the time required for stored water to drain:  
UBS Vol. (in.)  $\div$  Infiltration rate (in/hr) = Drain time (hrs)  
( recommend < 48 hrs)

# Pervious Paving

---

- Approach to Sizing Pervious Paving

- Self-Retaining

- Check the depth of the WQD volume in base/subbase:  
UBS volume (in.)  $\div$  0.40 = Depth (in.)

- Example: UBS volume = 1.0 in., depth = 2.5 in.  
(Minimum depth for vehicular traffic is 10 in.)

- Check the time required for stored water to drain:  
UBS Vol. (in.)  $\div$  Infiltration rate (in/hr) = Drain time (hrs)  
( recommend < 48 hrs)

# Sizing Rainwater Harvesting Cisterns

## ■ Rainwater Harvesting and Use

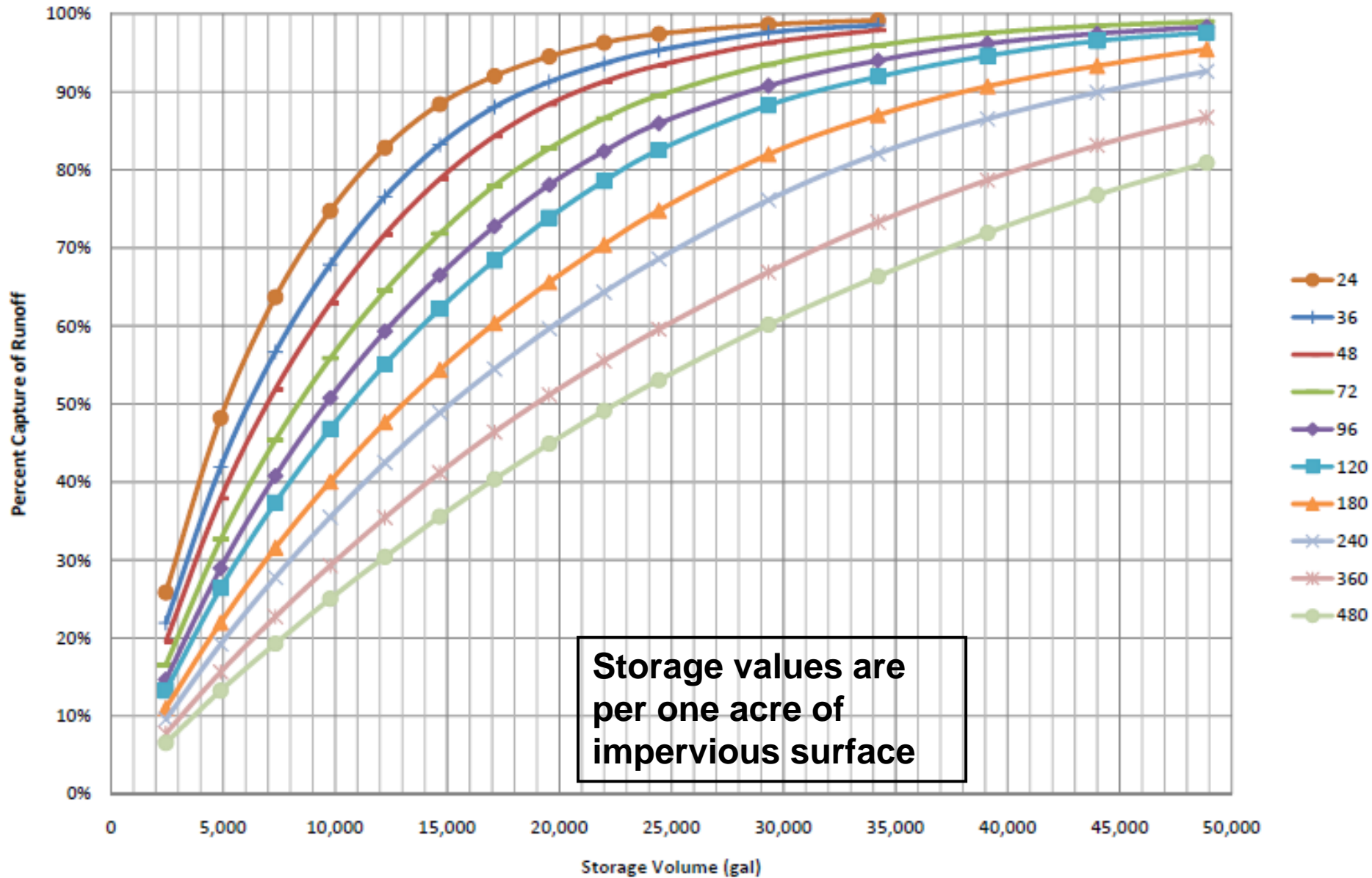
- Types of Demands

- Irrigation
- Toilet flushing
- Other non-potable



- Volume based sized criteria in C.3.d is 80% capture of the annual runoff
- Key concept is drawdown time
- Barriers: lack of plumbing codes, treatment, recycled water preference

**Figure G-9: Percent Capture Achieved by BMP Storage Volume for Various Drawdown Times - San Jose**



**Storage values are per one acre of impervious surface**

# Estimate Actual Demand

## Daily Use Rates for Toilets and Urinals<sup>1</sup>

Land Use Type	User Unit	User Unit Factor <sup>2</sup>	Daily Use/Unit (gal/day/unit)
Residential	Resident	2.9 residents per dwelling unit	8.6
Office or Retail	Employee (non-visitor)	200 SF per employee	6.9
Schools	Employee (not including students)	50 SF per employee	33.9

<sup>1</sup>References: CCCWP Stormwater C.3 Guidebook, 6<sup>th</sup> edition, 2012; BASMAA LID Feasibility Report, 2011; California Plumbing Code, 2010.

<sup>2</sup>Use project-specific data if available

# Example: 2-story Office Building



# Screening Worksheet Results

---

- Potential rainwater capture area = area of one building roof = 10,000 SF
- Convert to acres:  
 $10,000 \text{ SF} \div 43,560 \text{ SF/acre} = 0.23 \text{ acres}$
- Demand for commercial building:  
Interior floor area = 20,000 SF
- Minimum floor area to meet toilet flushing demand = 70,000 SF per acre of impervious surface
- Minimum floor area for this project to meet demand =  $70,000 \text{ SF/ac} \times 0.23 \text{ acres} = 16,100 \text{ SF}$
- $20,000 \text{ SF} > 16,100 \text{ SF} \Rightarrow$  Building will have minimum toilet flushing demand



# Determine Building Toilet Flushing Demand

---

- Building interior floor area = 20,000 SF
- Estimate no. of employees:
  - $200,000 \text{ SF} \div 200 \text{ SF/employee} = 100 \text{ employees}$
  - $100 \text{ employees} \times 6.9 \text{ gpd/employee} = 690 \text{ gpd}$
- Convert to equivalent demand per impervious acre (to allow use of sizing curves):
  - $10,000 \text{ SF roof area} \div 43,560 \text{ SF/ac} = 0.23 \text{ ac.}$
  - $690 \text{ gpd} \div 0.23 = 3,000 \text{ gpd per impervious acre}$

# Determine Required Cistern Size

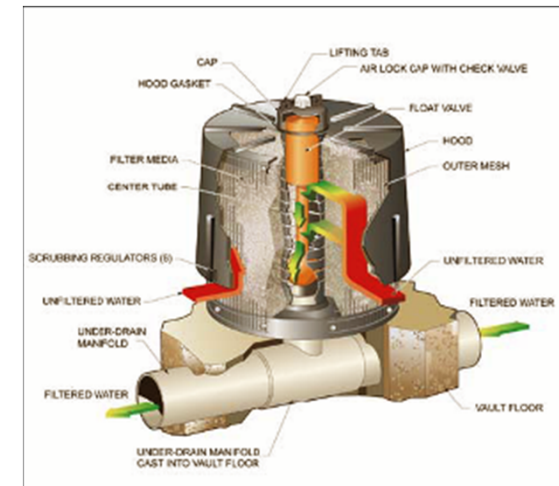
---

- From sizing curves, find right combination of drawdown time, tank size and required demand:
  - 480-hr (20-day) drawdown  $\Rightarrow$  49,000 gallon tank  $\Rightarrow$  2,450 gpd
  - 360-hr (15-day) drawdown  $\Rightarrow$  40,000 gallon tank  $\Rightarrow$  2,667 gpd
  - 240-hr (10-day) drawdown  $\Rightarrow$  32,000 gallon tank  $\Rightarrow$  3,200 gpd
  - 288-hr (12-day) drawdown  $\Rightarrow$  36,000 gallon tank  $\Rightarrow$  3,000 gpd  $\checkmark$
- Adjust tank size back to actual impervious area:
  - 36,000-gallon tank per 1 acre impervious area
  - $36,000 \times 0.23 \text{ acres} = \underline{8,300\text{-gallon tank}}$

# Sizing Non-LID Components

## Media Filters (cartridge type)

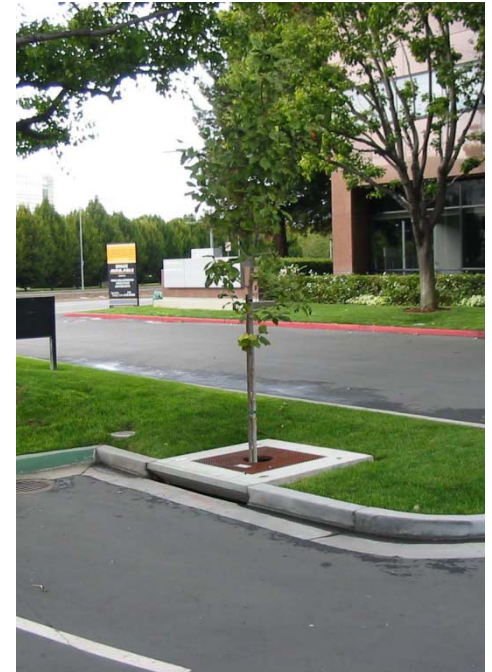
- Flow-based Treatment Measure
- Determine  $Q_{BMP}$
- From manufacturer's specifications, determine the design flow rate per cartridge
- Divide  $Q_{BMP}$  by the cartridge flow rate to calculate the number of cartridges required (round up)



# Sizing Non-LID Components

## High Flow Rate Tree Box Filters

- Flow-based Treatment Measure
- Determine  $Q_{BMP}$
- From manufacturer's specifications, determine the appropriate size of unit or combination of units
- A tree box filter that uses bio-treatment soil can be sized like a bioretention area or flow-through planter



# Sizing Non-LID Components

## Detention Basin

- Volume-based Treatment Measure (can only be used in treatment train)
- Determine  $V_{BMP}$
- Design outlet for 48-hour detention time
- If sizing for hydromodification management, use Bay Area Hydrology Model to determine size to meet HM standards



# ??? Questions ???

---

## Contact Information:

**Jill Bicknell**

**408-720-8811, X 1**

**[jcbicknell@eoainc.com](mailto:jcbicknell@eoainc.com)**

**[www.scvurppp.org](http://www.scvurppp.org)**

