CHAPTER 10, APPENDIX D

HYDROLOGIC AND HYDRAULIC CHECKLISTS

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HYDROLOGY AND HYDRAULICS QU	ALITY ASSURANCE CH	IECKLIST SUMMARY
PROJI	ECT DETAILS	
Road:		
Waterbody:		
District:		
Municipality:		
County:		
H&H Report Sealed by Licensed Engineer: District or Company:		
СН	ECKLISTS	
Checklists Completed Designer(s)	Reviewer(s)	Date(s)
H&H Report Abbreviated Full Hydrology HEC-RAS HY-8 Scour*		

Instructions:

- 1. These checklists are intended to provide documentation that a quality assurance review was performed. All applicable checklists must be completed by an internal reviewer and included with H&H Report submission. If the report is submitted as a paper copy for review, the completed QA checklists must be attached to the transmittal letter. If the report is uploaded to the JPA₂ Expert System for review, these completed QA forms must be placed in the "PennDOT Files" section of JPA₂ Expert. These forms are not intended to be transmitted to PADEP with the permit submission. Information stored in the "PennDOT Files" section of JPA₂ Expert will not be transferred to PADEP with the permit application.
- 2. The check boxes on the right side of the H&H Report checklists are used to indicate whether an item has been included. If the item is not required or does not apply to the particular project, check N/A.
- 3. When filling out the forms electronically, the individual sheet headings are automatically updated based on information from the summary sheet input.
- 4. Additional space for comments is provided in the last tab; please indicate the applicable QA sheet and section.
- Printing Instructions: When the applicable checklists have been completed, select those worksheets and select file print. (To select multiple worksheets, Hold the shift key and select the worksheet tabs at the bottom of the page). The page numbers will automatically be updated to correspond to the total number of pages printed.

Notes:

The summary sheet should be printed and submitted with the applicable checklists. Depending on the project type, not all checklists will be required for an H&H Report submission. For example, a small culvert replacement project may include the H&H Report, Hydrology and HY-8 checklists (unless HEC-RAS was used). Whereas a bridge replacement project may require the H&H Report, Hydrology, HEC-RAS and Scour checklists.

ABBREVIATED HYDROLOGIC AND HYDRAULIC REPORT CHECK	LIST		
Project: District:			
Municipality: County:			
Reviewer(s): Date:			
			ENT?
DESCRIPTION	YES	NO	N/A
B.1.a. LOCATION MAP			
Acceptable forms (one required):			-
USGS quadrangle map (or map of equal detail) page			
Aerial photographs page			
B.1.b. ENVIRONMENTAL CONCERNS			
1. PA Code Chapter 93 stream classification (check all that apply) page WWF CWF MF TSF HQ* EV*			I
*Note if HQ or EV Stream, Antidegredation analysis may be required - see DM2, Chapter 13.7 2. PA Fish and Boat Classification (check all that apply) Approved Trout Stream (stocked) Verified Natural Reproduction None			Ι
Type of material in stream bed from site inspection (i.e., sand, gravel, cobbles, etc.)			Ι
B.1.d. PHOTOGRAPHS page			
a. Existing structure (upstream and downstream face)			
b. Upstream / downstream channel and floodplain			
c. Past floods (if available)		ļ	
d. Roadway station ahead and station back (recommended)			
e. Photo location map (recommended)		L	l
Dates and other information relative to site inspection(s) made by designer date			Ι
B.2. HYDROLOGIC ANALYSIS			_
a. Show drainage area above proposed crossing (note method of page			I
determining area)			
b. Include design discharge(s) per Section 10.6.E page			
B 3 HYDRAULIC ANALYSIS			
a. The project is located in a FEMA mapped area? $\Box_{\text{ves}} \Box_{\text{no}}$			
If Yes is it a Detailed or Approximate area?			
(1) Original FIS study and flood map(s) provided page			
(2) Study is referenced in the text page			
(3) Was FEMA model obtained or documentation provided if unavailable?			
(4) Proposed structure encroaches on (check one): page			
 100-year floodplain (floodway fringe) 100-year floodway neither 			•
(5) Were existing flood elevations compared to FEMA's published? page			
(6) Were any differences in flood elevations > 0.5 ft explained? page			
b/c. Existing versus proposed conditions:			
(1) velocities* page			
(2) backwater elevations* page			
(3) bridge opening sizes (i.e., area of hydraulic openings) page		L	
(4) Is there an increase in the proposed 100-year flood elevation? $\Box_{yes} \Box_{no}$			
* Recommend including a table to compare all cross sections for the PennDOT design event and the 100-year event			

ABBREVIATED HYDROLOGIC AND HYDRAULIC F	REPORT CHECK	LIST	
Project:	District:		
Municipality:	County:		
Reviewer(s):	Date:		
DESCRIPTION		ITEM	NT?
c. Acceptable hydraulic methods for the site (check the method used)		123	IN/A
 HEC-RAS (bridge and culvert design, water surface profiles) HY-8 (culvert design) Other List: 			
 d. Estimated scour depths (refer to DM-4, Chapter 7) e. Riprap sizing for bank, pier, abutment, and/or culvert protection f. Construction measures (temp. stream crossings, causeways, roads, etc.) Comments or computations included 	page page page page		
 B.4. RISK ASSESSMENT OR ANALYSIS* Narrative description of factors related to the 100-year flood Narrative description of factors related to the 2-year flood (temporary conditions) * Refer to Section 10.7.C.4 for the definition and additional requirements of 	page page a risk analysis		
B.5. SUMMARY DATA SHEET Complete all information listed in the Summary Data Sheet (Figure 10.7.1) (available for download from http://www.dot.state.pa.us/hh/Summary-Data- Summary data matches the report tables, output/calculations, and TS&L	page Sheet.Zip)		
B.6. DRAWINGS AND FIGURES			
 a. Roadway plans and profiles indicating the following information: Locations of existing and/or proposed structures, stream channels and wetlands	page		
2. 100-year floodplain boundary	page		
 Temporary stream crossing, access road, cofferdam, diversion facility, etc. 	page		
 The magnitude, frequency and pertinent water surface elevation for PennDOT design and 100-year flood 	page		
 b. Plan drawing showing the location and orientation of all cross sections used in the hydraulic model (with scale, contours, and all important hydraulic features) 	page		
Cross-sections perpendicular to flood flow (minimum): Upstream (500 ft) Immediately upstream of proposed and/or existing crossings Immediately downstream of proposed and/or existing crossings Downstream (500 ft)	page		
is available in the HEC-RAS model submitted with the report	ormation		
 c. Profile of stream showing bed slope, normal water surface, and flood water surface elevations 	page		
 d. Cross section output of all cross sections used for backwater analysis e. Floodway maps and flood profiles from FEMA Flood Insurance Studies (when in a detailed FEMA study area) 	page page		
ELECTRONIC FILES			
Electronic files for the hydrologic and hydraulic models (as applicable)			

HYDRO	LOGIC AND HYDRAULIC REPOR	T CHECKLIST		
Project:		District:		
Municipality:		County:		
Reviewer(s):		Date:		
	DESCRIPTION		ITEM PRES	ENT?
C.1.a. LOCATION MAP				
Acceptable forms (one require USGS quadrangle Aerial photographs Required information: (1) Project location inc (2) Drainage area (3) Label stream and c	ed): map (or map of equal detail) s cluding proposed highway alignment direction, river reach studied	page page]
 C.1.b. EXISTING STRUCTURES (IF AF Identify existing hydraulic stru downstream of site Must describe:	PLICABLE) ctures (by map), including upstream and span lengths, pier orientation eath structure - stream clearance and skew structure locations with the proposed cross ctures are to remain in place	page , sing		
C.1.c. FLOOD INFORMATION		page		
 Elevations of available highwa Critical flood elevations of inte Local testimony of flooding (if per Section 10.7.C.1.i 	ater marks along the stream w/ dates of occ erest (possible damage) available) or structure performance (non-flo	currence ooding)		
C.1.d. ENVIRONMENTAL CONCERNS				
 PA Code Chapter 93 stream of WWF TSF *Note if HQ or EV Stream, An PA Fish and Boat Classification Approved Trout Stream of Verified Natural Restricted Natural Restrieted Natural Restrict	Classification (check all that apply) CWF MF HQ* EV* tidegredation analysis may be required - see on (check all that apply) ream (stocked) Class A Wild Trop eproduction None ental concerns mittent stream?	page ee DM2, Chapter 13.7 ut page]]]
C.1.e. HISTORY OF DRIFT, ICE AND S - Stability of stream banks (i.e., - Type of material in stream bea - History of ice accumulation or	TREAM BANK STABILITY exposed soil, slumping, tilting trees, etc.) d from site inspection (i.e., sand, gravel, co damage	page bbles, etc.)]
C.1.f. PHOTOGRAPHS		page		
 Existing structure (upstream a Upstream / downstream chan Past floods (if available) Roadway station ahead and s Photo location map (recomme Upstream and downstream st 	and downstream face) nel and floodplain atation back (recommended) anded) ructures			
C.1.g. FACTORS AFFECTING WATER	STAGES	page		
 High water from other streams Reservoirs (existing or propos Flood control projects and sta Other controls 	s sed) and approximate date of construction tus (e.g., control structures, operator, opera	ating policy)		

HYDROLOGIC AND HYDRAULIC REPORT CHECKLIST

			UNEIOT			
Proje	ect:	District	:			
Municipa	lity: (County	r:			
Reviewer((s):	Date	:			
	DESCRIPTION			ITEM	PRESE	NT?
				YES	NO	N/A
C.1.n. DEBRIS	e if debris can be a problem at the structure site	nade				
indicat		page				
C.1.i. SITE INSP	ECTION RECORDS	page				
- Dates	and other information relative to site inspection(s) made by designer	date				
- If appli	cable, documentaion of local testimony is included					
	GRADE APPROVAL	nage				
Indicat	e date of Line and Grade Approval or if pending	date		l I		
					I	
C.2. HYDROLOG	SIC ANALYSIS					
a. Show o	drainage area above proposed crossing (note method of	page				
b List flo	od records available	nage			_	
c. Include	e design discharge(s) per Section 10.6.E	page				
d. Show f	flood-frequency curve for the site	page				
e. Show s	stage-discharge-frequency curves for the site (existing and	page				
propos	ed conditions)					
C.3. HYDRAULI						
a. The pr	oject is located in a FEMA mapped area?	yes	no			
	If Yes is it a Detailed or Approximate area?					
	(1) Original FIS study and flood map(s) provided	page				
	 (2) Study is referenced in the text (3) Was EEMA model obtained or documentation provided if upavail 	page				
	(4) Proposed structure encroaches on (check one):	bade				
	100-year floodplain (floodway fringe)	1-0-				
	100-year floodway neither					
	(5) Were existing flood elevations compared to FEMA's published?	page				
h Existin	(b) were any differences in flood elevations > 0.5 it explained?	page				
D. EXISTIN	(1) velocities*	page				
	(2) backwater elevations*	page				
	(3) bridge opening sizes (i.e., area of hydraulic openings)	page				
	(4) Is there an increase in the proposed 100-year flood elevation?	yes	no			
	for the PennDOT design event and the 100-year event					
c. Accept	table hydraulic methods for the site (check the method used)			1		
	HEC-RAS (bridge and culvert design, water surface profiles)					
	HY-8 (culvert design) HDS-5 (culvert design - equival	ent to	HY-8)			
	HEC-2 (water surface profiles) USPRO (only if FEMA map rev	ision n	ecessary)			
	Other List:					
d. Was th	he HEC-RAS or HY-8 checklist completed?		_			
e. Model	validation	page				
	(1) Calibration with high water marks, storm events, and local testim	ony	_	└────┤		
f Cationa	(2) Explanation of model warnings and errors	0000		┝───┤		
a. Rinran	sizing for bank, pier, abutment, and culvert protection	page				
h. Constr	uction measures (temp. stream crossings, causeways, roads, etc.)	page				
	Supporting model or calculations included	page				
1						

HYDROLOGIC AND HYDRAULIC F	REPORT CHECKLIST
Project:	District:
Municipality:	County:
Reviewer(s):	Date:
DESCRIPTION	ITEM PRESENT?
	YES NO N/A
Narrative description of factors related to: - 100-year flood - overtopping flood - 2-year flood for temporary conditions	page
Refer to Section 10.7.0.4 for the definition and additional requi	irements of a risk analysis
C.5. SUMMARY DATA SHEET Complete all information listed in the Summary Data Sheet (Figu (available for download from http://www.dot.state.pa.us/hh/Sumr Summary data matches the report tables, output/calculations, ar	ure 10.7.1) page mary-Data-Sheet.Zip) nd TS&L
 C.6. DRAWINGS AND FIGURES a. Roadway plans and profiles indicating the following information: 1. Locations of existing and proposed structures, stream and wetlands Structure or culvert plan showing plan and elevation (Box culvert plans should show baffle layout) 2. Adjacent topographic features with key elevations or contours shown Profile drawing showing proposed structure and gro 3. 100-year floodplain boundary 4. Flood easement (if required) 5. Temporary stream crossing, access road, cofferdam, diversion facility, etc. 6. The magnitude, frequency and pertinent water surface elevations for specified floods b. Profile of stream showing the location and orientation of all cross see used for backwater analysis (with scale, contours, and all important hydraulic features) Cross-sections perpendicular to flood flow (minimum): Upstream (500 ft) Immediately downstream of proposed and/or existing component (500 ft) d. Floodway maps and flood profiles from FEMA Flood Insurance S (when in a detailed FEMA study area) 	n channels page II II II II I
ELECTRONIC FILES Electronic files provided for hydrologic & hydraulic models (as ap	pplicable)

HYDROLOGY CHECKLIST QUALITY ASSURANCE REVIEW			
Project: District:			
Municipality: County:			
Reviewer(s): Date:	1	1	1
DESCRIPTION	YES	NO	N/A
1. FEMA CONSIDERATIONS Is the proposed project in a detailed FEMA study area? If yes, are the following provided: - Published FIS flows page - Is FEMA hydrologic method acceptable per DM-2, Chapter 10? page - Are FEMA flows compared with calculated flows using PennDOT acceptable methods? - Is FEMA's published 100-year flow included in the analysis?			
2. ACT 167 Is there a DEP approved Act 167 Stormwater Management Plan? How were the flows developed in the Act 167? Were there flows provided in the vicinity of the project site? Have the flows been included for comparison to calculated flows? page Comments:			
3. DESIGN FLOODS PennDOT roadway classification PennDOT design event (check one) 10-yr PADEP event (check one) 25-yr (rural) So-yr (suburban) 100-yr (urban) Comments:]
4. HYDROLOGIC ANALYSIS]

		HYD	ROLOGY CH	IECKLIST			
		QUALI	TY ASSURAN				
Project:				District:			
Municipality:				County:			
Reviewer(s):				Date:			
		DESCRI	PTION		YES	NO	N/A
METHOD SELI	ECTION DETAIL	S					
ill out the approp	riate section belo	w based on the hyd	rologic method(s) used in Section 4.			
A. WRCI	Method (gage)			page			
	USGS gage #			-			
	Gage location (i	.e, town and stream	/river name)				I
	Gage is on the s	same main stem as	the project site				
	Print out of gage	e record is included					
	DA at gage			square miles			I
	DA _{site} is betwee	n 0.5 and 1.5 DA _{gage}	e				
	Years of record						I
	Record is greate	er than 10 years					
	Historic peaks (i.e., not recorded by	gage) are exclud	ded			
	Record not part	ially influenced by re	egulation or diver	sion (e.g., reservoir, levee, etc.)			
	Watershed char	racteristics consister	nt for entire recor	d (e.g., landuse)			
	Skew calculatio	n method is appropr	riate (check one):				
		Station					
		Regional					
		Weighted					Ē
	If gage is not at	project site, were flo	ows correctly tran	islated to the site?			
	Return Period (yrs)	Q _{gage} (cfs)	Q _{site} (cfs)]			
	,						
				1			
				1			
]			
Commen	ts:						

			W			
Project:	QUALITI ASSURAL		District:			
Municipality			County:			
Reviewer(s):			Date:			
	DESCRIPTION		2 0101	YES	NO	I
B. Ratio	nal Method		page			t
	DA is less than 200 acres		P 3			t
	Weighted C value is correct C =	-				1
	Time of concentration (T_c) is correct T_c :	=				1
	Storm duration equals the T _c for intensity determina	tion				T
	Rainfall intensity from PDT-IDF curves					
	Return Period (yrs) Intensity (in) Q (cfs)					
		-				
Commen	ts:					
Commen	ts: 		page			I
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles		page			Ī
Commen	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one)	A	page	-		Ŧ
Commen C. USGS	WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable	A	page □ B %			
Commen	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable	A	раде В % %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5%	A	раде В% %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable	A	раде В % %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable	A	page □ B % % %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable Return Period Q (cfs)	A	раде В% % %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable Return Period Q (cfs)	A	page □ B % % %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable $\frac{eturn Period}{(yrs)} Q (cfs)$	A	раде В% % % %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable Return Period Q (cfs) Q (cfs) Q (cfs)	A	раде В % % %			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable $\frac{eturn Period}{(yrs)} Q(cfs)$	A	page			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable $\frac{eturn Period}{(yrs)} Q(cfs)$	A	page			
Commen C. USGS	ts: WRIR 2000-4189 Method DA at site between 1.5 and 2,000 square miles Region is correct (check one) % Forest is reasonable % Urban is reasonable In Region B, urban % does not exceed 5% % Carbonate is reasonable % Controlled is reasonable $ \overline{\text{Return Period} \ Q(cfs)} $ ts:	A	page			

	QUALITY ASSUR				
Project:		District:			
Municipality:		County:			
Reviewer(s):		Date:			
	DESCRIPTION		YES	NO	N/A
D. USGS SIR 2008-51	02 Method	page			
DA at site be	tween 1.0 and 2,000 square miles				
Region is co	rrect (check one)	1 2 3 4			
% Forest is r	easonable	%			
% Urban is r	easonable	%			
% Carbonate	e is reasonable	%			
% Storage is	reasonable*	%			
*surface area	a of lakes, ponds, wetlands, etc.				
Mean basin	elevation is is correct (Region 3)				
Comments:	Q (cfs)				

District:			
County:			
Date:			
	YES	NO	N//
page			
1 2 3 4			
feet			
%			
	District: County: Date: page 	District: County: Date: YES page 1 2 3 4 feet %	District: County: Date: YES NO page 1 2 3 4 feet %

		QUAL	TT ASSURAN				
Project:				District:			
Municipality:				County:			
Reviewer(s):				Date:			
		DESCR	IPTION		YES	NO	N/A
F. TR-55	Method / WinTr-	55		page			
	DA at site is betw	ween 10 and 2,000	acres (< 3.1 squ	are miles) for TR-55			
	DA at site is betw	ween 1 acre and 2	5 square miles for	WinTR-55			
	Note if multiple of	drainage areas are	used, attach add	itional sheets for CN, etc.			
	CN calculated co	orrectly	CN =				
	Time of concent	ration (T _c) calculate	ed correctly	hrs (0.1 <t<sub>c<10 hr)</t<sub>			
	Sheet flow lengt	h no greater than 1	00'	feet			
	Shallow concent	trated flow length a	ppropriate	feet			
	Channel flow ler	ngth appropriate		feet			
	Rainfall from PDT-IDF curves (24-hour duration)						
	PDT-IDF Curve	or SCS Type II 24-	hr rainfall distribu	tion used			
						-	-
	Return Period (yrs)	Rainfall (in)	Q (cfs)				
				1			
				1			
				1			
				1			
				1			
				-			
Comment	s:						

		QUALI	IT ASSURAN				
Project:				District:			
Municipality:				County:			
Reviewer(s):				Date:			
		DESCR	IPTION		YES	NO	N/A
G. EFH2 N	lethod			page			
I	DA at site betwe	en 1 and 2,000 acr	res (< 3.1 square	miles)			
(CN calculated co	orrectly	CN =				
I	Urban % does n	ot exceed 10%		%			
I	Hydraulic length	is between 200 an	d 26,000 feet	feet			
	Average watersh	ned slope, Y		%			
`	Y is the average	overland slope bet	tween drainage d	vide and stream channel			
1	Rainfall from PD	T-IDF curves (24-h	our duration)				1
1	PDT-IDF Curve	or SCS Type II 24-I	hr rainfall distribut	ion used			
		,					
[Return Period (yrs)	Rainfall (in)	Q (cfs)				
F							
F							
_							
L				1			
Commonts							
Comments							

		QUALI	IT ASSURAN					
Project:				District:				
Municipality:				County:				
Reviewer(s):				Date:				
		DESCRI	IPTION			YES	NO	
H. HEC-1 / HE	C-HMS Met	hod		page				
DA s	ubareas ≤ 3	3.1 square miles or	justification provi	ded for larger subarea	s			
Note	if multiple of	drainage areas are	used, attach add	tional sheets for CN, e	etc.			
CN c	alculated co	orrectly	CN =					
Lag	time, t _L			hours	;			
Lag	time calcula	ted with SCS metho	od					
lf su	bdivided, ro	uting was performe	d					
Rain	fall from PD	T-IDF curves (24-h	our duration)					
PDT	-IDF Curve	or SCS Type II 24-h	nr rainfall distribu	ion used				1
Ret	urn Period (yrs)	Rainfall (in)	Q (cfs)					
			•					
Comments:								
					-			

HYDROLOGY CHECKLIST QUALITY ASSURANCE REVIEW								
Project:				District:				
Municipality:				County:				
Reviewer(s):				Date:				
		DESCRI	PTION		YES	NO	N/A	
I. Other M	ethod			page	_			
	Calculations incl	uded						
	Method appropri	iate for location						
	Rationale / justif	ication provided						
	Return Period (yrs)	Rainfall (in)	Q (cfs)					
]				
Comment	s:							

	HEC-R	AS MODEL CHECKLIST	
	QUALIT	TY ASSURANCE REVIEW	
	Project:	District:	
	Municipality:	County:	
	Reviewer(s):	Date:	
1.	File Management** HEC-RAS program version Project file name (*.prj) Plan name for existing conditions Plan Short ID Geometry file name (*.gxx) Steady flow file name (*.fxx) Final date of run file (*.rxx) Plan name for proposed conditions Plan Short ID Geometry file name (*.gxx) Steady flow file name (*.fxx) Final date of run file (*.rxx) Plan name for temp conditions (if applicable) Plan Short ID Geometry file name (*.gxx) Steady flow file name (*.fxx) Final date of run file (*.rxx) Plan name for temp conditions (if applicable) Plan Short ID Geometry file name (*.gxx) Steady flow file name (*.fxx) Final date of run file (*.rxx) **The following HEC-RAS files must be submit plan (*.pxx), run (*.rxx), and output (*.oxx). The plan file extension. Comments:	tted for review: project (*.prj), geometry (*.gxx), steady flow (*.fxx) e run file and output file extensions will correspond to the appropri	
	DESCH	RIPTION YES	NO N/A
	 Hydraulic model used in the FEMA study Was the original FEMA model obtained? Paper copy of model input Paper copy of model output Electronic files If FEMA modeling data was unavailable, Datum: FEMA Project Datum Conversion (FEMA to project): List the FEMA cross sections used as-is 	<pre>/ check all that apply) page / (check all that apply) page page letter from FEMA stating such is provided? ctft in existing conditions model</pre>	
	- List the FEMA cross sections modified w	rith current survey in existing conditions model	
	Does the hydraulic cross section plan sho surveyed sections used in the existing co	ow all FEMA sections and page	_

	HEC-RAS MODEL CHECKLIST QUALITY ASSURANCE REVIEW			
	Project: District:			
	Municipality: County:			
	Reviewer(s): Date:		-	-
	DESCRIPTION	YES	NO	N/A
3.	Steady Flow Data Boundary Conditions Upstream Downstream Normal depth S= S= Known WS WS Elev= WS Elev= Critical depth Critical depth Source= Rating curve source= source= - Are the boundary conditions appropriate? - Are the same boundary conditions used in the existing and proposed models? - If applicable, was a known WS used for the FEMA published flow? Discharge Information (see also Hydrology checklist) - - 100-year, DEP and PennDOT design events were modeled - year - Flows for the modeled events match peak flows in the H&H Report - Flow change(s) reflects tributary location(s) Comments:			
4.	Geometric Data Plan Information / River System Schematic - Plan showing the location and orientation of all cross sections provided (with scale, contours, and all important hydraulic features) page			
		=		
		—		

	HEC-RAS MODEL CHECKLIST			
	QUALITY ASSURANCE REVIEW			
	Project: District:			
	Municipality: County:			
	Reviewer(s): Date:			
	DESCRIPTION	YES	NO	
	Geometric Data continued			.
ı.	Bridge Geometry*			
	- Plan with high/low chord elevations included page			
	- Bridge cross section (E) (P)			
	- Bounding bridge sections are at or beyond the embankment toe and parallel to each other			1
	- High chord (max.) (E) (P)			-
	- Low chord (min.) (E) (P)			
	- High/low chords match the report/drawings			1
	- Bridge width (E) (P)			-
	- Bridge widths match the report/drawings			1
	- Distance to US section (E) (P)			
	- US distances match the hydraulic section plan			1
	- Number of spans (E) (P)			
	- Normal clear span length(s)			
	- Bridge normal clear span lengths match the report/drawings			1
	- Number of piers (E) (P)			
	- Existing pier centerline(s), width(s) and elevation(s) are correct			Г
	- Proposed pier centerline(s), width(s) and elevation(s) are correct			
	- Ineffective areas "turn off" when weir flow passes over bridge			
	- Minimum weir flow elevation is reasonable			1
	- Bridge modeling methods Existing Proposed			
	Low flow			
	High flow	-		
	- Methods are appropriate per Reference 1	-		1
				1
	* Check for existing (E) and proposed (P) structure; low chord elevations and normal clear span			
	lengths are not applicable to arch structures.			
	Configure and the balance of an original configure of			
	Comments:			
		-		
		-		
		-		
		-		

	HEC-RAS MODEL CHECKLIST QUALITY ASSURANCE REVIEW								
	Project:	District:							
	Municipality:	County:							
	Reviewer(s):	Date:	_						
	DESCRIPTION		YES	NO	N/A				
	Geometric Data continued			1	-				
4b.	*Culvert Geometry Plan with inverts elevations included	220							
		page			J				
	- Structure cross section (E)	(P)							
	- Bounding culvert cross sections are at or beyond the embankment toe]				
	- Ineffective areas "turn off" when weir flow passes over road								
	- Minimum weir flow elevation is reasonable]				
	Existing	Proposed							
	- Number of Darreis								
	- Shape								
	Span x Rise								
	- Spans/diameters are correct			I	1				
	- Chart #			-	3				
	- Scale #				_				
	 Chart and Scale match the culvert type and entrance conditions 								
	- Distance to US sectionft	ft		r	1				
	- US distances match the hydraulic section plan	4]				
	- Culvert lengths match the hydraulic section and structure plans	IL		r	1				
	- Entrance loss coeff				J				
	- Exit loss coeff								
	- Loss coefficients are appropriate for entrance/exit conditions			I	1				
	- Manning's n for top			-	-				
	- Manning's n for bottom				_				
	 Manning's n for top and bottom are appropriate 				l				
	- Depth to use bottom n								
	- Depth blocked			r	<u>г</u>				
	- Blocked depth reflects the depressed depth for fish passage	ft							
	- DS invert elevation ft	n							
	- Invert elevations match the report/drawings	n		1	1				
	- High chord (max.) ft	ft			1				
	- High chords match structure drawings]				
	* Check for existing (E) and proposed (P) structure								
	• • • •								
	Comments:								

	HEC-RAS MODEL CHECKLIST			
	QUALITY ASSURANCE REVIEW			
	Project: District: Municipality: County: Reviewer(s): Date:			
		VES	NO	N/A
	DESCRIPTION	TES	NO	N/A
4c.	*Roadway Profile - Roadway profile plan provided - Roadway stations are entered from left to right (looking downstream) - Roadway (high chord) stations and elevations match drawings (exist and prop) - Highest roadway elevation is coded as the US side so that weir flow is correctly calculated.			
	* Check for existing (E) and proposed (P) structure			
	Comments:			
4d.	Temporary Conditions - Temporary fill and/or structure(s) proposed in the channel? (check all that apply)			
5.	Plan File Flow Regime Subcritical Mixed			
	 If subcritical only, is the Froude number < 1.0 at every section? If supercritical only, is the Froude number > 1.0 at every section? 			
	Comments:			

	HEC-RAS MODEL CHECKLIST				
	QUALITY ASSURANCE REVIEW				
Project:		District:			
Municipality:		County:			
Reviewer(s):		Date:			
	DESCRIPTION		YES	NO	
Output Existing versus Proposed (- Water surface profile: - Is the existing low cho	Dutput s are in the correct order in the cross section output ord elevation equal to or below the proposed?				1
- Hydraulic opening are	ea stated(sf) Exist	(sf) Prop.			
- Is the proposed open	ing area equal to or larger than the existing?				
- Errors, warnings, and	I notes reviewed and discussed	page			
- Are mere increases a	d HEC-RAS cross section plot output are included	nage			ł
- Existing and propose	d HEC-RAS profile output are included	page			
(table & plot)		P90		<u> </u>	
- Output shows 100-ye	ar, DEP and PennDOT design events				
Comments:					
Temporary Conditions Out - The H&H report state - The magnitude and e - Are the temporary inc - Do the temporary was	put is that theyear event does not overtop the extent of temporary increases are quantified preases contained within the channel? els tie in to existing wsels within the study limits?	temporary measures page page page			
Comments:					

References

¹ Hydrologic Engineering Center. 2002. HEC-RAS, River Analysis System Hydraulic Reference Manual. U.S. Army Corps of Engineers, Davis, CA. ² Hydrologic Engineering Center. 1995. RD-42, Flow Transitions in Bridge Backwater Analysis, U.S. Army Corps of Engineers,

Davis, CA.

HY-8 MODEL CHECKLIST			
QUALITY ASSURANCE REVIEW			
Project:	District:		
Municipality:	County:		
Reviewer(s):	Date:		
DESCRIPTION		YES	NO
1. File Management			
Project file name			
Is HY-8 Run (input/output) attached?			
Commentes			
Comments:			
2. Discharge Date (Creasing Dreparties)			
Elows Input:			
Minimumcfs			
Designcfs			
Maximumcts			
100-year and DEP's & PennDOT's design events were modeled			
Flows for the modeled events are correct			
Commentes			
comments.			
2 Tailwater Data (Crossing Properties)			
Channel Type (check one)			
Rectangular Irregular			
Trapezoidal Rating Curve			
Triangular Constant Tailwater Elevation			
Channel type selection is reasonable			
Channel input dimensions are consistent with plans			
Commentes			
Comments.			
A Readway Data (Crossing Proportion)			
4. Roadway Data (Clossing Properties) Roadway Profile Shape (check one)			
Constant			
Roadway profile dimensions are consistent with plans			
Comments:			

HY-8 MODEL CHECKLIST		
QUALITY ASSURANCE REVIEW		
Project: District:		
Municipality: County:		
Reviewer(s): Date:		
DESCRIPTION	YES	NO
5a. Existing Culvert Data (Culvert Properties) Culvert Name Shape (check one) Circular Arch-Open Bottom Concrete Box Low Profile Arch Elliptical High Profile Arch Pipe Arch Metal Box User Defined Arch-Box-Concrete Culvert material and size:		
Sb. Proposed Curvent Data (Curvent Properties) Culvert Name Shape (check one) Circular Arch-Open Bottom Concrete Box Low Profile Arch Elliptical High Profile Arch Pipe Arch Metal Box User Defined Arch-Box-Concrete Culvert material and size:		
Culvert specifications are consistent with plans		
Comments:		
6. Site Data (Culvert Properties) Site Data Input Option (check one) Culvert Invert Data Embankment Toe Data		
Site data is consistent with plans		
Comments:		

HY-8	MODEL CHECKLIST		
QUALIT	Y ASSURANCE REVIEW		
Project:	District:		
Municipality:	County:		
Reviewer(s):	Date:		
DESC	RIPTION Y	ES	NO
7. Results			
Overtopping?			
If yes, overtopping discharge:			
Existing	cts		
Proposed	cts		
Upstream 100-year water surface elevation	а.		
Existing	H #		
FTOPUSEU	n areater than existing?	- T	
		I	
Design storm velocity			
Existing	ft/s		
Proposed	ft/s		
100-year flood velocity			
Existing	ft/s		
Proposed	ft/s		
Results are acceptable for HY-8 use			
Entrance velocities < 5 fps			
Comments:			
Federal Highway Administration (FHWA) HY-8 Version 7.0, Marc	ch 16, 2007		
Software developed by: Environmental Modeling Systems, Inc.			
Based on HDS-5 Documentation			

	SCOUR ANALYSIS & RIPRAP SIZING CHECKLIST						
	QUALITY ASSURANCE REVIEW						
Projec	t: District:						
Municipali	ty: County:						
Reviewer(s							
	DESCRIPTION	YES	NO	N/A			
. Strean	nbed Particle Size						
	$-D_{50}$ II = II						
	Clay and silt = 0.00024 - 0.062 mm						
	Sand 0.062 - 2.00 mm 0.002 - 0.08 in						
	Gravel 2 - 64 mm 0.08 - 2.5 in						
	Cobbles 64 - 250 mm 2.5 - 10 in						
	- Method used to determine D ₅₀						
	visual inspection isieve analysis						
	☐ pebble count ☐ core boring* (see notes on applicability below)			-			
	- Location of streambed sample						
	- Streambed material description page						
	- Is bedrock visible?						
	- Is D ₅₀ appropriate for studied reach?						
* For limitations on core borings use see requirements in DM-4, Chapter 7. Also for most							
PA streams core borings may underestimate the size of the streambed material. If the							
	approximate D_{100} particle size is less than the core diameter and the sample is taken in						
C	the stream channel, the core borings may provide a reasonable D ₅₀ for the armor layer.						
Comm	ents:						
Contra	action Scour						
HEC-R	AS Sections - fill in the appropriate information from the proposed HEC-RAS model						
	YS						
	Length = ft						
	XS:						
	XS: Length = ft						
	<u> </u>						
	1. Upstream uncontracted cross section (XS output)						
	2. Internal bridge cross section (BR U or BR D in HEC-RAS output)						
	3. Upstream bounding cross section (XS output)						
Comm	ents:						

Project: District: Aunicipality: County: eviewer(s): Date: DESCRIPTION Contraction Scour (continued) Critical Velocity - Was HEC-18, Equation 5.1 used? - K _u coefficient is correct (6.19 - SI units / 11.17 - English units) - y is channel hydraulic depth variable from XS (1) - V is channel velocity from XS (1) - 100-year scour type (check one) Clear Live - 500-year scour type (check one) Clear Live - HEC-RAS output tables are included with input parameters labeled page	YES	NO	N/
Aunicipality: County: eviewer(s): Date: DESCRIPTION Contraction Scour (continued) Critical Velocity - Was HEC-18, Equation 5.1 used? - K_u coefficient is correct (6.19 - SI units / 11.17 - English units) - y is channel hydraulic depth variable from XS (1) - - V is channel velocity from XS (1) - - 100-year scour type (check one) Clear Live - 500-year scour type (check one) Clear Live - HEC-RAS output tables are included with input parameters labeled page	YES	NO	N /
eviewer(s): Date: DESCRIPTION Contraction Scour (continued) Critical Velocity - Was HEC-18, Equation 5.1 used? - K _u coefficient is correct (6.19 - SI units / 11.17 - English units) - y is channel hydraulic depth variable from XS (1) - V is channel velocity from XS (1) - V is channel velocity from XS (1) - Olear - 100-year scour type (check one) Clear - 500-year scour type (check one) Clear - HEC-RAS output tables are included with input parameters labeled page Comments:	YES	NO	N/
DESCRIPTION Contraction Scour (continued) Critical Velocity • Was HEC-18, Equation 5.1 used? Ku coefficient is correct (6.19 - Sl units / 11.17 - English units) y is channel hydraulic depth variable from XS (1) Vis channel velocity from XS (1) Vis channel velocity from XS (1) Vis channel velocity from XS (1) Vis channel velocity from XS (1) Vis channel velocity from XS (1) Vis channel velocity from XS (1) Vis channel velocity from XS (1) Live - HEC-RAS output tables are included with input parameters labeled page Comments:	YES	NO	N/.
Contraction Scour (continued) Critical Velocity - Was HEC-18, Equation 5.1 used? - K _u coefficient is correct (6.19 - SI units / 11.17 - English units) - y is channel hydraulic depth variable from XS (1) - V is channel velocity from XS (1) - 100-year scour type (check one) - 600-year scour type (check one) - HEC-RAS output tables are included with input parameters labeled page - HEC-RAS output tables are included with input parameters labeled page Comments:]
Critical Velocity - Was HEC-18, Equation 5.1 used? - K _u coefficient is correct (6.19 - SI units / 11.17 - English units) - y is channel hydraulic depth variable from XS (1) - V is channel velocity from XS (1) - 100-year scour type (check one) Clear - Live - 500-year scour type (check one) Clear - Live - HEC-RAS output tables are included with input parameters labeled page Comments: Live-Bed Scour - calculate for the event(s) determined to be live-bed* *Where coarse sediments are present, it is recommended that scour depths be calculated for live bed scour conditions using the clear-water and live-bed equations, and the smaller scour depth be used Were HEC-18, Equations 5.2-5.3 used? - y ₁ is channel hydraulic depth variable from XS (1) - y ₀ is hydraulic depth variable from XS (2) - W ₁ (check one) - M ₁ is the estimated bottom or top channel width from XS (1) - W ₂ (check one) - Muther the work with the work with the model with from XS (2) - W ₁ is the estimated bottom or top channel width from XS (2) - W ₂ is the estimated bottom or top channel width from XS (2)]
- Was HEC-18, Equation 5.1 used? - K _u coefficient is correct (6.19 - SI units / 11.17 - English units) - y is channel hydraulic depth variable from XS (1) - V is channel velocity from XS (1) - 100-year scour type (check one) □ Clear □ Live - 500-year scour type (check one) □ Clear □ Live - HEC-RAS output tables are included with input parameters labeled page]
K _u coefficient is correct (6.19 - SI units / 11.17 - English units) y is channel hydraulic depth variable from XS (1) V is channel velocity from XS (1) O is channel velocity tables are included with input parameters labeled page Comments: Description Descri]
 y is channel hydraulic depth variable from XS (1) V is channel velocity from XS (1) 100-year scour type (check one) Clear Live 500-year scour type (check one) Clear Live HEC-RAS output tables are included with input parameters labeled page]
 V is channel velocity from XS (1) 100-year scour type (check one)]
- 100-year scour type (check one) Clear Live - 500-year scour type (check one) Clear Live - HEC-RAS output tables are included with input parameters labeled page Comments: Coddddddddddddddddddddddddddddddddd]
- 500-year scour type (check one)ClearLive - HEC-RAS output tables are included with input parameters labeled page Comments:]
- HEC-RAS output tables are included with input parameters labeled page Comments:			J
Comments:			
Live-Bed Scour - calculate for the event(s) determined to be live-bed* *Where coarse sediments are present, it is recommended that scour depths be calculated for live bed scour conditions using the clear-water and live-bed equations, and the smaller scour depth be used. • Were HEC-18, Equations 5.2-5.3 used? • y ₁ is channel hydraulic depth variable from XS (1) • y ₀ is hydraulic depth variable from XS (2) • W ₁ (check one) ft • W ₁ is the estimated bottom or top channel width from XS (1) • W ₂ is the estimated bottom or top channel width from XS (2)			
Live-Bed Scour - calculate for the event(s) determined to be live-bed* *Where coarse sediments are present, it is recommended that scour depths be calculated for live-bed scour conditions using the clear-water and live-bed equations, and the smaller scour depth be used. - Were HEC-18, Equations 5.2-5.3 used? - y ₁ is channel hydraulic depth variable from XS (1) - y ₀ is hydraulic depth variable from XS (2) - W ₁ (check one) - W ₁ is the estimated bottom or top channel width from XS (1) - W ₂ (check one) - W ₂ is the estimated bottom or top channel width from XS (2)			
Live-Bed Scour - calculate for the event(s) determined to be live-bed* *Where coarse sediments are present, it is recommended that scour depths be calculated for live-bed scour conditions using the clear-water and live-bed equations, and the smaller scour depth be used. - Were HEC-18, Equations 5.2-5.3 used? - y ₁ is channel hydraulic depth variable from XS (1) - y ₀ is hydraulic depth variable from XS (2) - W ₁ (check one) - W ₁ is the estimated bottom or top channel width from XS (1) - W ₂ (check one) - W ₂ is the estimated bottom or top channel width from XS (2)			
Live-Bed Scour - calculate for the event(s) determined to be live-bed* *Where coarse sediments are present, it is recommended that scour depths be calculated for live-bed scour conditions using the clear-water and live-bed equations, and the smaller scour depth be used. - Were HEC-18, Equations 5.2-5.3 used? - y ₁ is channel hydraulic depth variable from XS (1) - y ₀ is hydraulic depth variable from XS (2) - W ₁ (check one) - W ₁ is the estimated bottom or top channel width from XS (1) - W ₂ (check one) - W ₂ is the estimated bottom or top channel width from XS (2)			
Live-Bed Scour - calculate for the event(s) determined to be live-bed* *Where coarse sediments are present, it is recommended that scour depths be calculated for live-bed scour conditions using the clear-water and live-bed equations, and the smaller scour depth be used. - Were HEC-18, Equations 5.2-5.3 used? - y ₁ is channel hydraulic depth variable from XS (1) - y ₀ is hydraulic depth variable from XS (2) - W ₁ (check one) ft - W ₂ (check one) - W ₂ is the estimated bottom or top channel width from XS (2) - W ₂ is the estimated bottom or top channel width from XS (2)			
- Were HEC-18, Equations 5.2-5.3 used? - y_1 is channel hydraulic depth variable from XS (1) - y_0 is hydraulic depth variable from XS (2) - W_1 (check one) ft Top Bottom - W_1 is the estimated bottom or top channel width from XS (1) - W_2 (check one) ft Top Bottom - W_2 is the estimated bottom or top channel width from XS (2)			
- y_1 is channel hydraulic depth variable from XS (1) - y_0 is hydraulic depth variable from XS (2) - W_1 (check one) ft Top Bottom - W_1 is the estimated bottom or top channel width from XS (1) - W_2 (check one) ft Top Bottom - W_2 is the estimated bottom or top channel width from XS (2)			
- y_0 is hydraulic depth variable from XS (2) - W_1 (check one) ft Top Bottom - W_1 is the estimated bottom or top channel width from XS (1) - W_2 (check one) ft Top Bottom - W_2 is the estimated bottom or top channel width from XS (2)			
- W_1 (check one) ft Top Bottom - W_1 is the estimated bottom or top channel width from XS (1) - W_2 (check one) ft Top Bottom - W_2 is the estimated bottom or top channel width from XS (2)]
- W_1 is the estimated bottom or top channel width from XS (1) - W_2 (check one) ft Top Bottom - W_2 is the estimated bottom or top channel width from XS (2)		1	1
- vv ₂ (cneck one)ft Top Bottom - W ₂ is the estimated bottom or top channel width from XS (2)			
- vv_2 is the estimated bottom or top channel width from XS (2)		1	1
- w_1 and w_2 are consistent (both top or both bottom)			
- Q_1 is the channel flow from XS (1)			
- Q_2 is the flow in the contracted channel ^{**} from XS (2)			
- κ_1 coefficient correct			
(0.59 - mostly contact, 0.64 - some suspended, 0.69 - mostly suspended)			
$- y_{s} (100-yr \text{ event}) \qquad \qquad$			
$- y_s (500-yr event)$ ft		1	1
- HEC-RAS output tables are included with input parameters labeled page			
**If the proposed bridge abutments are located in the channel (HEC-18, Case 1a) or at the channel banks (HEC-18, Case 1b), Q ₂ should be the flow through the bridge opening.			
Comments:			

SCOUR ANALYSIS & RIPRAP SIZING CHECKLIST QUALITY ASSURANCE REVIEW Project: District: Municipality: County: Reviewer(s): Date: DESCRIPTION YES NO N/A Clear-Water Scour - calculate for the event(s) determined to be clear-water - Were HEC-18, Equations 5.4-5.5 used? - K_u coefficient is correct (0.025 - SI units / 0.0077 - English units) - y_o is hydraulic depth variable from XS (2) W (check one) ft 🗌 Top Bottom - W is the bottom or top channel width from XS (2) - Q is the flow through the bridge opening or on the set-back over bank area at the bridge associated with the width, W, from XS (2) - HEC-RAS output tables are included with input parameters labeled page Comments: 3. Local Pier Scour (if applicable) Local Pier Scour for Simple Piers - Was HEC-18, Equation 6.3 used? - Pier nose shape - K₁ pier nose coefficient is correct (HEC-18, Table 6.1) - Angle of attack of flow, θ (θ is 0 when the pier is aligned with the flow direction) - K₂ angle of attack coefficient is correct (HEC-18, Table 6.2) - K₃ bed condition coefficient is correct (HEC-18, Table 6.3) - K₄ armoring factor coefficient is correct (values less than 1 require a sieve analysis) - y_1 is hydraulic depth directly upstream of the pier from XS (3) flow distribution table - V₁ is velocity directly upstream of the pier from XS (3) flow distribution table - g, acceleration of gravity (check one) 9.81 m/s² 32.2 ft/s² - Fr₁ is the Froude number directly upstream of the pier from XS (3) $Fr_1 = V_1 / (gy_1)^{0.5}$ - a, pier width ft - a is the pier width perpendicular to the flow direction (i.e., projected pier width) - K_w calculated with Eqn. 6.9 or 6.10 if y/a < 0.8, a/D₅₀ > 50, and Fr < 1 - y_s (100-yr event) ft - y_s (500-yr event) ft - HEC-RAS output tables are included with input parameters labeled page Comments:



SCOUR	ANALYSIS	& RIPRAP	SIZING	CHECKLIST
	QUALITY /	ASSURAN	CE REVI	EW

QUALITY ASSURANCE REVIEW					
Project	: District:				
Municipality					
Reviewer(s)	: Date:			-	
	DESCRIPTION	YES	NO	N/A	
Total S	cour				
	- If live-bed contraction scour depths are limited by streambed armoring,				
	was the lesser of the clear-water and live-bed contraction scour depths used?				
	- If multi-layered riprap protection is proposed for the piers, was the local				
	pier scour depth reduced by 50%?				
	- If the structure has piers, was the total pier scour depth calculated as the				
	sum of the contraction scour, pressure scour, and adjusted local pier scour depths?				
	- Scour envelope is illustrated on the HEC-RAS bridge section page				
	- Total scour depths are included in the H&H Report page				
	- If any aggradation or degradation was indicated in bridge inspection reports				
	was it included with total scour?		ļ		
	- Scour depths were calculated for the temporary bridge (25-year event) per DM-4, Chap 5.				
*Note: I	Per DM-4, Chapter 7 local abutment scour calculations are not required				
when th	ne substructure is protected with multi-layered riprap protection.				
Comme	ents:				
Riprap	Sizing				
	- Unfactored velocities				
	Abutment Piers				
	V ₁₀₀ ft/s V ₁₀₀ ft/s				
	V ₅₀₀ ft/s V ₅₀₀ ft/s				
	- For abutments, V is the BR Open Vel variable for the velocity inside the bridge				
	- For piers, V is the avg upstream velocity in the section upstream of the piers - XS (3)				
	- HEC-RAS bridge output table shows inside bridge velocity page				
	- What event has the highest velocity inside the bridge?				
	- Was the highest velocity used?**				
Abutme	ents				
	- Was the 1.8 safety factor applied to the velocity <u>before</u> sizing the riprap?		<u> </u>		
	- Riprap size meets DM-4 Chapter 7 requirements R			J	
Piers				1	
	- Was the 1.5 safety factor applied to the velocity <u>before</u> sizing the riprap?			<u> </u>	
	- Riprap size meets DM-4 Chapter 7 requirements R				
Tempoi	rary Bridge				
	- Was the 1.8 safety factor applied to the 25-year velocity per DM-4, Chapter 5?				
	- Riprap size meets DM-4 Chapter 7 requirements R				
**Note: when th highest	Per DM-4 Chapter 7, riprap has to be designed to withstand the 500-year velocity only the 500-year scour depth is below the bottom of footing elevation. If a lower event has the velocity inside the bridge, it should be used for riprap sizing.				
Comm	ents:				
highest Comme	velocity inside the bridge, it should be used for riprap sizing.				

US Department of Transportation, FHWA. Hydraulic Engineering Circular No. 18 (HEC-18), Evaluating Scour at Bridges, 4th Edition. May 2001.

	ADDITIONAL COMMENTS			
QUALITY ASSURANCE REVIEW				
Project:	District:			
municipality.	County.			
Checklist	Section			
Checklist	Section			
Chacklist	Section			
Checklist	Section			
Checklist	Section			
Checklist	Section			
Checklist	Section			